

U is the friction curvature coefficient. If left blank, the default value of 0.20 will be used. A separate value may be input for each span.

K is the friction wobble coefficient per foot of stressing steel. If left blank, a default value of 0.0 will be used. A separate value may be input for each span.

fs is the ultimate strength of the prestressing steel in kips per square inch. If left blank, a default value of 270 ksi will be used. Only one value may be input for each path.

Specifications												P-Jack				f'c		Losses				
u X10 <sup>2</sup>		k X10 <sup>-4</sup>			fs KSI		% Jack	End	Anchor Set		Allow. Tension		KIPS	Low Relax.	KSI	% Shortening	KSI					
46	47	48	49	50	51	52			53	LT. 1/8 in.	RT. 1/8 in.	Spec %						%	64	65	66	67

%JACK is the maximum allowable temporary jacking stress expressed as a percent of the ultimate strength of the prestressing steel. If left blank, a default value of 75% is used. Only one value may be input for each path. If both end stressing is requested, the same value for %JACK is used at each end.

END is the input to request jacking location. Enter "L" for left end jacking, enter "R" for right end jacking, enter "B" for both end jacking. If left blank, a default value of "B" is used. Only one value may be entered for each path.

ANCHOR SET, LT, and RT is the length, in eighths of an inch, of the anchor set. If one end jacking was requested, no entry need be made for the anchor set of the non-jacked end. Only one value for anchor set left, and one value for anchor set right may be entered per path. If left blank, a default value of 5/8" is used.

ALLOWABLE TENSION is the given value of the allowable maximum tension stress for which the frame is to be designed. Two methods of input are provided. If a check (✓) is placed in the SPEC input field, the allowable tension will be calculated using the specifications in the Bridge Planning and Design Manual, Volume I. The formula is  $6\sqrt{f'c}$ .

If an entry is made in the % field, the allowable tension is entered % times  $6\sqrt{f'c}$ . Only one choice is allowed and only one entry is allowed per path. If left blank, the program will design for no tension (if possible).

P-JACK is the input prestress force in kips. If a value for P-JACK is input for all paths in a given trial and frame, then the program only analyzes the structure and reports the effects. If multiple path prestressing is described, all values of P-JACK except one must be given. The path with no value will have its prestress force designed by the program based on full DL + added DL + LL + I. Stresses are then checked for both the DL + PS and DL + added DL + PS cases to assure that tension in these cases does not exceed zero. If tension is detected, P-JACK is redesigned to eliminate the tension and a warning message is printed. When this occurs, the full allowable tension value may not appear in the final stresses.

$f'c$  is the required concrete strength. This value is used as the basis for calculating the allowable tension. If left blank, a default value of 3.5 ksi will be used.

% SHORTENING is the percent of theoretical elastic shortening to be included in the prestressing calculations based on the final prestress force coefficients. Only those frames with columns affected by shortening will be considered. If left blank, a default value of 100% is used. To eliminate shortening, enter a zero. To obtain any other even 10% increment enter its multiple of 10%.

Losses (ksi) are the losses due to creep and shrinkage. If left blank, a default value of 32 ksi is used.

## RESULTS

The following items are listed as they appear in the output for each problem. Whether a particular item appears, depends on the nature of the problem.

First, the FRAME DESCRIPTION input is reported as given, except for the deflection and sidesway entries, which do not appear.

SECTION PROPERTIES are reported for each section described with the Section Properties by Parts input option. The input is reported as given, followed by the area, centroid location with respect to the X Y coordinate system chosen, and the moments of inertia about the centroidal X-X and Y-Y axes.

When the above sections supplement a frame member, MEMBER PROPERTIES about the X-X axis are reported. They are the length of the member, minimum moment of inertia about the X-X axis, and the relative stiffness (small k) and carry over factors.

The absence on any message under FRAME DIAGNOSTICS, indicates that no errors have been detected in the data which makes up the frame. The presence of an error message terminates processing of the problem. If the first diagnostic states that 'Errors have been found in either the frame description or cross section data', this indicates that error messages have been printed somewhere in the preceding output of the problem. Although processing is terminated, Load Data input is reported as given, along with error messages pertaining to it.

FRAME PROPERTIES is the result of combining the Frame Description with the Member Properties. Carry Over and Distribution Factors are adjusted for pinned end conditions. At this point, all data pertaining to the frame should be reviewed to determine if it is reasonable and describes the frame as intended.

For each trial, the LOADINGS are reported as given. Errors, if any, in Load Data are reported, and result in processing being terminated for the trial.

FIXED END MOMENTS are the total FEM's for a trial and are adjusted for pinned end conditions.

Distributed MOMENTS and SHEARS are reported at the one-tenth points for each member. In each case, the horizontal members are separated from the vertical members. Beam sign convention is used.

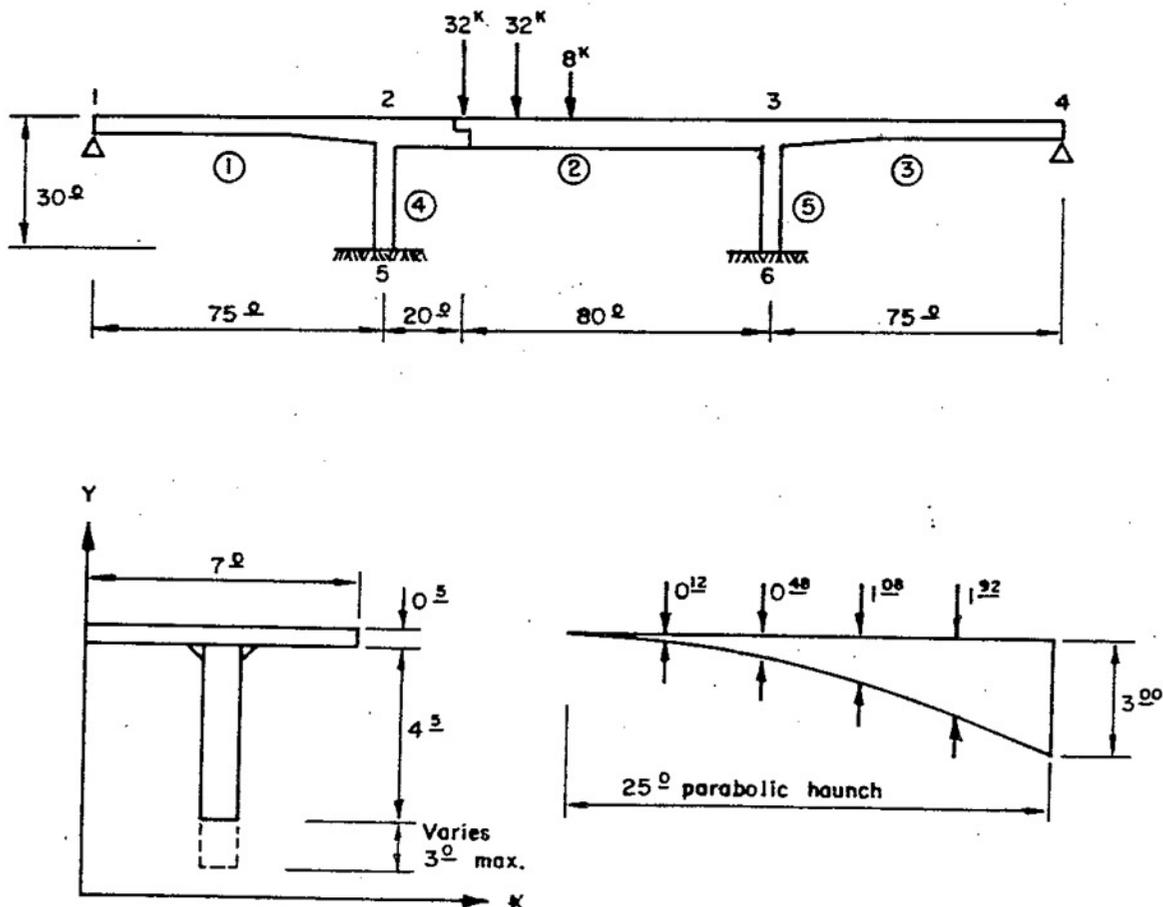
COLUMN REACTIONS are calculated on the assumption that the user followed the rule that the left end of the column was the bottom. Any deviation from this rule will yield incorrect reactions.

The TANGENTIAL ROTATIONS at the ends of each member are measured from the unloaded position, with clockwise being positive.

DEFLECTIONS are measured from the unload position. Positive is downward for horizontal members and to the right for vertical members.

### EXAMPLE

Following is an example problem intended to illustrate the main features of the program.



# FRAME SYSTEM

## Frame Description

Page 1 of 3

Name Example 1

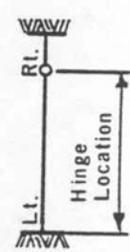
Phone \_\_\_\_\_

IDENT.	SOURCE	CHARGE	EXPENDITURE	SPECIAL DESIGNATION	PROGRAM
DIST.	GR.	UNIT	DIST.	UNIT	NUMBER
<u>14T</u>	<u>2000</u>	<u>1403314033910002</u>			<u>B D E φ 3 5</u>
S/C 1201, 2488					

Member No.	End Joint No.		Length ft.	Min. I ft. <sup>4</sup>	Hinge Location ft.	E ksi	Dead Load		Member Properties			Recall		D.L. Sidesway		
	Lt.	Rt.					Uniform k/ft.	Unit Wt. pcf	-K- Stiffness Factor	-C- Carry Over Factor	Member	Reverse	Deflections			
							Lt.	Rt.	Lt.	Rt.	Lt.	Rt.				
1	2	3	750		200		180	150								
2	3	4	1000				180	150								
3	4	5	300	1000			600									
4	5	6	300	1000			600									
5	6															
6																

END CONDITION:  
 C = Cantilever  
 P = Pin  
 R = Roller

DIRECTION:  
 G or H = Horizontal



S/C 2488

# FRAME SYSTEM Section Properties By Parts

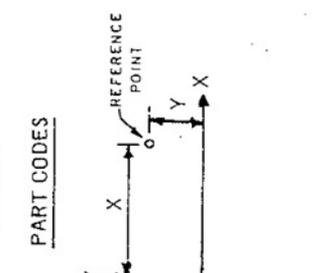
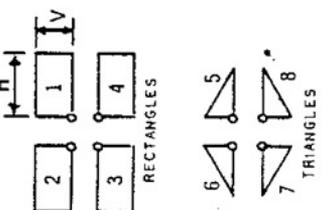
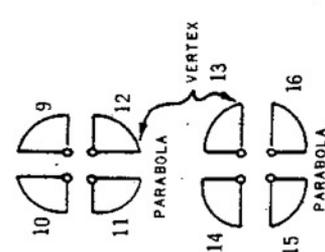
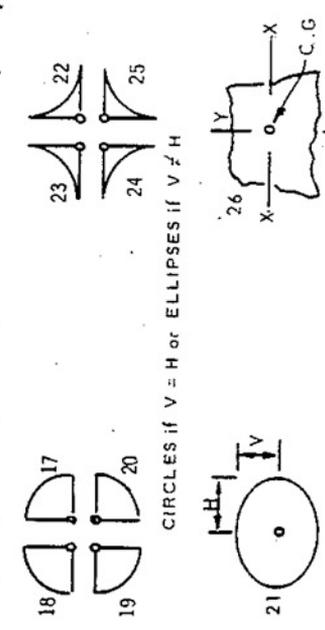
Page **2** of **3**

Name **Example 1**

Phone

IDENT.	CHARGE	SPECIAL DESIGNATION	PROGRAM
LIST GROUP BATCH/PROB	DISF. UNIT	WHEN APPLICABLE	NUMBER
<b>14-T 2000</b>			
S/C 1201, 2489			B D E / 3 5

Member No.	Cross Section Location	Recall	Sign + or -	Part Code	Part Dimensions		Ref. Pt. Coord.		Area ft. <sup>2</sup>	Any Shape		Store
					Vertical V ft.	Horizontal H ft.	X ft.	Y ft.		I <sub>xx</sub> ft. <sup>4</sup>	I <sub>yy</sub> ft. <sup>4</sup>	
01	00			01	50	700	00	750				
				01	450	100	300	300				
				07	33	33	300	750				
				08	33	33	400	750				
				04	48	100	300	300				
					108							
					192							
					300							
02	00	02										02



S. C. 2489

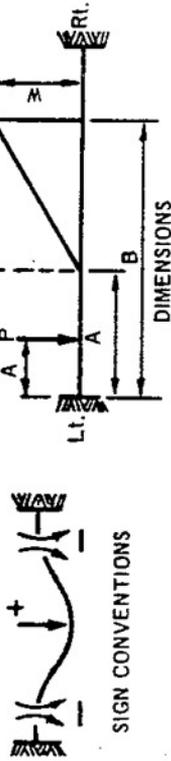
# FRAME SYSTEM

## Load Data

Page **3** of **3**

Name **Example 1**

Phone



**\* CODE**  
 L = Max. W on left  
 R = Max. W on right  
 U = Uniform Load  
 P = Point load.

IDENT. DIST. GR. BATCH PROB.  
**14T 2000**  
 S/C 2491

Trial No.	Member No.	Loads			FEMs*		Deflections	Sidesway	Comments
		W or P ± k/ft. or k	A * Code ft.	B ft.	Left ± ft-k	Right ± ft-k			
0002	↙	25200P	00						CAP. WT.
↙	↙	21000P	200						HINGE WT.
0102	↙	25200P	1000				1.0		CAP. WT.
↙	↙	32000P	200						TRUCK LOAD
↙	↙	32000P	340						1. LANE
		8000P	480						NB. IMPACT

S/C 2491

\* When FEMs are given, they are not calculated for any load on that member.

IDENT		FRAME SYSTEM										JUN. 06, 1969		PAGE 1			
FRAME DESCRIPTION																	
END																	
CARRY OVER																	
MEM	JT	JT	COND		DIR		SPAN	I	HINGE	E	DEAD LOAD		K		FACTORS		RECALL
NO	LT	RT	LT	RT							UNIFORM	SEC	LT	RT	LT	RT	MEM
1	1	2	P	G	75.0	0.0	0.0	0.0	0.0	0.180	150.0	0.0	0.0	0.0	0.0	0.0	
2	2	3		G	100.0	0.0	20.0	0.0	0.180	150.0	0.0	0.0	0.0	0.0	0.0	0.0	
3	3	4			0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	DIR
4	5	2			30.0	10.00	0.0	0.0	0.600	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
5	6	3			30.0	10.00	0.0	0.0	0.600	0.0	0.0	0.0	0.0	0.0	0.0	0.0	

IDENT		FRAME SYSTEM										JUN. 06, 1969		PAGE 2		
SECTION PROPERTIES																
MEM	LOC	RECALL	SIGN	CODE	V	H	X	Y	AREA	INERTIAS		STORE				
										IXX	IYY					
1	0.0			1	4.50	1.00	3.00	3.00	0.0	0.0	0.0					
1	0.0			1	0.50	7.00	0.0	7.50	0.0	0.0	0.0					
1	0.0			8	0.33	0.33	4.00	7.50	0.0	0.0	0.0	01				
1	0.0			7	0.33	0.33	3.00	7.50	0.0	0.0	0.0					
AREA					CENTROID LOCATION			MOMENT OF INERTIA ABOUT CENTROID								
					X	Y	X-X	Y-Y								
8.11					3.50	6.36	20.09	14.71								

MEM	LOC	RECALL	SIGN	CODE	V	H	X	Y	AREA	INERTIAS		STORE
										IXX	IYY	
1	50.0	01		0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
RECALL		1										
AREA					CENTROID LOCATION			MOMENT OF INERTIA ABOUT CENTROID				
					X	Y	X-X	Y-Y				
8.11					3.50	6.36	20.09	14.71				

MEM	LOC	RECALL	SIGN	CODE	V	H	X	Y	AREA	INERTIAS		STORE
										IXX	IYY	
1	60.0	01		4	0.48	1.00	3.00	3.00	0.0	0.0	0.0	
RECALL		1										
AREA					CENTROID LOCATION			MOMENT OF INERTIA ABOUT CENTROID				
					X	Y	X-X	Y-Y				
8.59					3.50	6.36	25.97	14.75				

IDENT		FRAME SYSTEM										JUN. 06, 1969		PAGE 3		
SECTION PROPERTIES																
MEM	LOC	RECALL	SIGN	CODE	V	H	X	Y	AREA	INERTIAS		STORE				
										IXX	IYY					
1	65.0	01		4	1.00	1.00	3.00	3.00	0.0	0.0	0.0					
RECALL		1														
AREA					CENTROID LOCATION			MOMENT OF INERTIA ABOUT CENTROID								
					X	Y	X-X	Y-Y								
9.19					3.50	5.90	34.67	14.80								

MEM	LOC	RECALL	SIGN	CODE	V	H	X	Y	AREA	INERTIAS		STORE
										IXX	IYY	
1	70.0	01		4	1.92	1.00	3.00	3.00	0.0	0.0	0.0	
RECALL		1										
AREA					CENTROID LOCATION			MOMENT OF INERTIA ABOUT CENTROID				
					X	Y	X-X	Y-Y				
10.03					3.50	5.53	49.62	14.87				

MEM	LOC	RECALL	SIGN	CODE	V	H	X	Y	AREA	INERTIAS		STORE
										IXX	IYY	
1	75.0	01		4	3.00	1.00	3.00	3.00	0.0	0.0	0.0	02
RECALL		1										
AREA					CENTROID LOCATION			MOMENT OF INERTIA ABOUT CENTROID				
					X	Y	X-X	Y-Y				
11.11					3.50	5.05	74.02	14.96				

MEMBER 1 PROPERTIES						
LENGTH	MIN INERTIA		STIFFNESS		CARRY OVER	
	LT	RT	LT	RT	LT	RT
75.0	20.09		4.367	6.066	0.955	0.471

IDENT 14T 20 00		FRAME SYSTEM				JUN. 06, 1969				PAGE 4		
SECTION PROPERTIES												
MEM	LOC	RECALL	SIGN	CODE	V	H	X	Y	AREA	IXX	IYY	STORE
2	0.0	02	0		0.0	0.0	0.0	0.0	0.0	0.0	0.0	
	RECALL	2					3.50	5.05	11.11	74.02	14.96	
AREA		CENTROID LOCATION				MOMENT OF INERTIA ABOUT CENTROID						
			X	Y		X-X		Y-Y				
	11.11		3.50	5.05		74.02		14.96				
MEMBER 2 PROPERTIES												
HINGE AT LOCATION 20.0												
LENGTH		MIN INERTIA		STIFFNESS		CARRY OVER						
				LT	RT	LT	RT	LT	RT			
100.0		74.02		0.231	3.692	4.000	0.250					

IDENT 14T 20 00		FRAME SYSTEM				JUN. 06, 1969				PAGE 5			
FRAME DIAGNOSTICS													
FRAME PROPERTIES													
MEM	JT	JT	COND	END					CARRY OVER		DISTRIBUTION		
NO	LT	RT	LT	RT	DIR	SPAN	I	HINGE	E	LT	RT	LT	RT
1	1	2	P	G		75.0	20.09	0.0	750.	0.655	0.0	0.0	0.428
2	2	3		G		100.0	74.02	20.0	750.	4.000	0.250	0.065	0.527
3	3	4	P	G		75.0	20.09	0.0	750.	0.0	0.655	0.216	0.0
4	5	2				30.0	10.00	0.0	750.	0.500	0.500	0.0	0.507
5	6	3				30.0	10.00	0.0	750.	0.500	0.500	0.0	0.507

IDENT 14T 20 00		FRAME SYSTEM				JUN. 06, 1969				PAGE 6	
LOADINGS TRIAL 0											
MEM	W OR P	LOAD	CODE	A	B	FIXED END MOMENTS					
						LEFT	RIGHT	DEFLT	COMMENTS		
2	21.000	P		20.0	0.0	0.	0.		HINGE WT.		
2	25.200	P		0.0	0.0	0.	0.		CAP WT.		
2	25.200	P		100.0	0.0	0.	0.		CAP WT.		
FIXED END MOMENTS TRIAL 0											
MEM	FIXED END MOMENTS		MEM	FIXED END MOMENTS		MEM	FIXED END MOMENTS				
NO	LT	RT	NO	LT	RT	NO	LT	RT			
1	0.	-1250.	2	-1869.	-1588.	3	-1250.	0.			
4	0.	0.	5	0.	0.						

IDENT 14T 20 00		FRAME SYSTEM				JUN. 06, 1969				PAGE 7	
HORIZONTAL MEMBER MOMENTS TRIAL 0											
MEM	LEFT	.1 PT	.2 PT	.3 PT	.4 PT	.5 PT	.6 PT	.7 PT	.8 PT	.9 PT	RIGHT
1	0.	202.	325.	369.	336.	223.	32.	-238.	-587.	-1020.	-1544.
2	-1893.	-854.	0.	459.	734.	824.	730.	451.	-13.	-662.	-1495.
3	-1362.	-856.	-441.	-110.	141.	314.	408.	424.	361.	220.	0.
VERTICAL MEMBER MOMENTS TRIAL 0											
MEM	LEFT	.1 PT	.2 PT	.3 PT	.4 PT	.5 PT	.6 PT	.7 PT	.8 PT	.9 PT	RIGHT
4	174.	122.	70.	17.	-35.	-87.	-140.	-192.	-244.	-297.	-349.
5	-66.	-46.	-27.	-7.	13.	33.	53.	73.	93.	113.	133.
HORIZONTAL MEMBER SHEARS TRIAL 0											
MEM	LEFT	.1 PT	.2 PT	.3 PT	.4 PT	.5 PT	.6 PT	.7 PT	.8 PT	.9 PT	RIGHT
1	32.1	21.7	11.2	0.7	-9.8	-20.2	-30.7	-41.2	-52.0	-63.6	-76.5
2	138.3	94.6	76.2	36.7	18.2	-0.2	-18.7	-37.1	-55.6	-74.1	-117.7
3	74.1	61.1	49.6	38.8	28.3	17.8	7.3	-3.1	-13.6	-24.1	-34.6
VERTICAL MEMBER SHEARS TRIAL 0											
MEM	LEFT	.1 PT	.2 PT	.3 PT	.4 PT	.5 PT	.6 PT	.7 PT	.8 PT	.9 PT	RIGHT
4	-17.4	-17.4	-17.4	-17.4	-17.4	-17.4	-17.4	-17.4	-17.4	-17.4	-17.4
5	6.6	6.6	6.6	6.6	6.6	6.6	6.6	6.6	6.6	6.6	6.6

IDENT 14T 20 00 FRAME SYSTEM JUN. 06, 1969 PAGE 8

COLUMN REACTIONS TRIAL 0

MEM NO	LT REACTION	RT REACTION	COLUMN WEIGHT
4	232.8	214.8	18.0
5	209.8	191.8	18.0

IDENT 14T 20 00 FRAME SYSTEM JUN. 06, 1969 PAGE 9

TANGENTIAL ROTATIONS - RADIANS - CLOCKWISE POSITIVE

SPAN	LT. END	RT. END	SPAN	LT. END	RT. END	SPAN	LT. END	RT. END
1	0.002898	0.002422	2	0.002422	-0.000922	3	-0.000922	-0.003880
4	0.000000	0.002422	5	0.000000	-0.000922			

HORIZONTAL MEMBER DEFLECTIONS IN FEET AT 1/4 POINTS FROM LEFT END - DOWNWARD POSITIVE

MEMBER	F=	LT	1/4	1/2	3/4	RT
MEMBER 1	F= 750.	0.0	0.061	0.032	-0.008	0.0
MEMBER 2	F= 750.	0.0	0.086	0.099	0.055	0.0
LONG HINGE		LT	1/4	1/2	3/4	RT
		0.078	0.100	0.088	0.042	0.0
MEMBER 3	F= 750.	0.0	0.014	0.059	0.059	0.0

VERTICAL MEMBER DEFLECTIONS IN FEET AT 1/4 POINTS FROM LEFT END.

MEMBER	F=	LT	1/4	1/2	3/4	RT
MEMBER 4	F= 750.	0.0	-0.003	-0.009	-0.010	0.0
MEMBER 5	F= 750.	0.0	0.001	0.003	0.004	0.0

IDENT 14T 20 00 FRAME SYSTEM JUN. 06, 1969 PAGE 10

LOADINGS TRIAL 1

MEM	M OR P	LOAD CODE	A		B		FIXED END MOMENTS			COMMENTS
			A	B	LEFT	RIGHT	DEFLT			
2	8.000	P	48.0	0.0	0.	0.			NO IMPACT	
2	32.000	P	34.0	0.0	0.	0.			1 LANE	
2	32.000	P	20.0	0.0	0.	0.	10		TRUCK LOAD	

FIXED END MOMENTS TRIAL 1

MEM NO	FIXED END MOMENTS		MEM NO	FIXED END MOMENTS		MEM NO	FIXED END MOMENTS	
	LT	RT		LT	RT		LT	RT
1	0.	0.	2	-1175.	-389.	3	0.	0.
4	0.	0.	5	0.	0.			

IDENT 14T 20 00 FRAME SYSTEM JUN. 06, 1969 PAGE 11

HORIZONTAL MEMBER MOMENTS TRIAL 1

MEM NO	LEFT	.1 PT	.2 PT	.3 PT	.4 PT	.5 PT	.6 PT	.7 PT	.8 PT	.9 PT	RIGHT
1	0.	-54.	-109.	-163.	-217.	-271.	-326.	-380.	-434.	-488.	-543.
2	-1187.	-593.	0.	273.	355.	292.	166.	39.	-87.	-214.	-340.
3	-156.	-140.	-124.	-109.	-93.	-78.	-62.	-47.	-31.	-16.	0.

VERTICAL MEMBER MOMENTS TRIAL 1

MEM NO	LEFT	.1 PT	.2 PT	.3 PT	.4 PT	.5 PT	.6 PT	.7 PT	.8 PT	.9 PT	RIGHT
4	322.	225.	129.	32.	-64.	-161.	-258.	-354.	-451.	-548.	-644.
5	-92.	-65.	-37.	-9.	18.	46.	74.	102.	129.	157.	185.

HORIZONTAL MEMBER SHEARS TRIAL 1

MEM NO	LEFT	.1 PT	.2 PT	.3 PT	.4 PT	.5 PT	.6 PT	.7 PT	.8 PT	.9 PT	RIGHT
1	-7.2	-7.2	-7.2	-7.2	-7.2	-7.2	-7.2	-7.2	-7.2	-7.2	-7.2
2	59.3	59.3	59.3	27.3	-4.7	-12.7	-12.7	-12.7	-12.7	-12.7	-12.7
3	2.1	2.1	2.1	2.1	2.1	2.1	2.1	2.1	2.1	2.1	2.1

VERTICAL MEMBER SHEARS TRIAL 1

MEM NO	LEFT	.1 PT	.2 PT	.3 PT	.4 PT	.5 PT	.6 PT	.7 PT	.8 PT	.9 PT	RIGHT
4	-32.2	-32.2	-32.2	-32.2	-32.2	-32.2	-32.2	-32.2	-32.2	-32.2	-32.2
5	9.2	9.2	9.2	9.2	9.2	9.2	9.2	9.2	9.2	9.2	9.2

IDENT 147 20 00		FRAME SYSTEM		JUN. 06, 1969		PAGE 12	
COLUMN REACTIONS TRIAL 1							
MEM NO	LT REACTION	RT REACTION	COLUMN WEIGHT				
4	66.6	66.6					
5	14.7	14.7					

IDENT 147 20 00		FRAME SYSTEM		JUN. 06, 1969		PAGE 13		
TANGENTIAL ROTATIONS - RADIAN - CLOCKWISE POSITIVE								
SPAN	LT. END	RT. END	SPAN	LT. END	RT. END	SPAN	LT. END	RT. END
1	-0.002930	0.004474	2	0.004474	-0.001283	3	-0.001283	0.000840
4	-0.000000	0.004474	5	0.000000	-0.001283			
HORIZONTAL MEMBER DEFLECTIONS IN FEET AT 1/10 POINTS FROM LEFT END - DOWNWARD POSITIVE								
MEMBER 1	E= 750.	0.0	-0.022	-0.042	-0.060	-0.073	-0.081	
		-0.081	-0.073	-0.057	-0.031	0.0		
MEMBER 2	F= 750.	0.0	0.051	0.109	0.105	0.097	0.084	
		0.088	0.051	0.032	0.015	0.0		
LONG HINGE		LT	1/4	1/2	3/4	RT		
		0.109	0.097	0.068	0.032	0.0		
MEMBER 3	E= 750.	0.0	-0.009	-0.016	-0.021	-0.023	-0.023	
		-0.021	-0.017	-0.012	-0.006	0.0		
VERTICAL MEMBER DEFLECTIONS IN FEET AT 1/10 POINTS FROM LEFT END.								
MEMBER 4	E= 750.	0.0	-0.001	-0.004	-0.008	-0.013	-0.017	
		-0.019	-0.020	-0.017	-0.011	0.0		
MEMBER 5	F= 750.	0.0	0.000	0.001	0.002	0.004	0.005	
		0.006	0.006	0.005	0.003	0.0		

LIVE LOAD RESULTS

LIVE LOAD DIAGNOSTICS, if present, will indicate that the live load limitations placed on the frame have been violated or an error was made in the superstructure live load input data.

SUPERSTRUCTURE LIVE LOAD input data is reported as given or assumed.

For each L. L. NO. for which LIVE LOAD DATA was given or assumed, the following results are reported for the tenth points of the horizontal members.

1. NEGATIVE LIVE LOAD MOMENT ENVELOPE
2. DEAD LOAD PLUS NEGATIVE LIVE LOAD MOMENT ENVELOPE
3. POSITIVE LIVE LOAD MOMENT ENVELOPE
4. DEAD LOAD PLUS POSITIVE LIVE LOAD MOMENT ENVELOPE
5. LIVE LOAD SHEAR ENVELOPES  
(POSITIVE, NEGATIVE and RANGE)
6. DEAD LOAD PLUS LIVE LOAD SHEAR ENVELOPES (POSITIVE and NEGATIVE)

The dead load plus live load envelopes are reported only if the dead load analysis was performed with all horizontal members loaded. The dead load results are obtained from Trial No. 0 of the basic system as described in earlier instructions.

Be careful when using the dead load plus live load shear envelopes. The dead load shears are computed, and only one ordinate is saved at each tenth point. Therefore, when the dead load includes concentrated loads, the abrupt steps are not shown. If the load is exactly at a tenth point, only the most positive value is retained.

LIVE LOAD SUPPORT RESULTS are also reported for each L.L. NO. for which LIVE LOAD DATA was given or assumed. Dead load is not included. Impact is included, unless otherwise specified.

The MAX. POSITIVE AND NEGATIVE (uplift) AXIAL LOAD at each SUPPORT or TOP of VERTICAL MEMBER is reported, as is TOP and BOTTOM vertical member moments created by the same loading.

The MAX POSITIVE and NEGATIVE LONGITUDINAL MOMENT at the TOP of each VERTICAL MEMBER is reported, as is the AXIAL LOAD and BOTTOM vertical member moment created by the same loading.

Beam sign convention is used for all live load results. Units are kips and feet.

#### INFLUENCE LINE RESULTS

When requested, the following types of INFLUENCE LINES are reported. Ordinates for each are given at the tenth points and at hinges.

The INFLUENCE LINES for GIRDER MOMENT, reported for each tenth point of the horizontal members, are extended two spans on each side of the span with the influence point.

The INFLUENCE LINES for GIRDER SHEAR, reported for the left end of each horizontal member, are extended two spans on each side of the span with the influence point. From this line, the influence line for shear at any point in that member may be constructed.

The INFLUENCE LINES for REACTION at TOP of COLUMN (or support if no column is present) are extended two spans on each side of the column.

The INFLUENCE LINES for MOMENT at TOP of COLUMN are extended two spans on each side of the column.

#### MODIFICATIONS TO EXISTING SYSTEM

Due to popular demand, the following modifications were made to the existing system. Details of the basis system are described in earlier instructions.

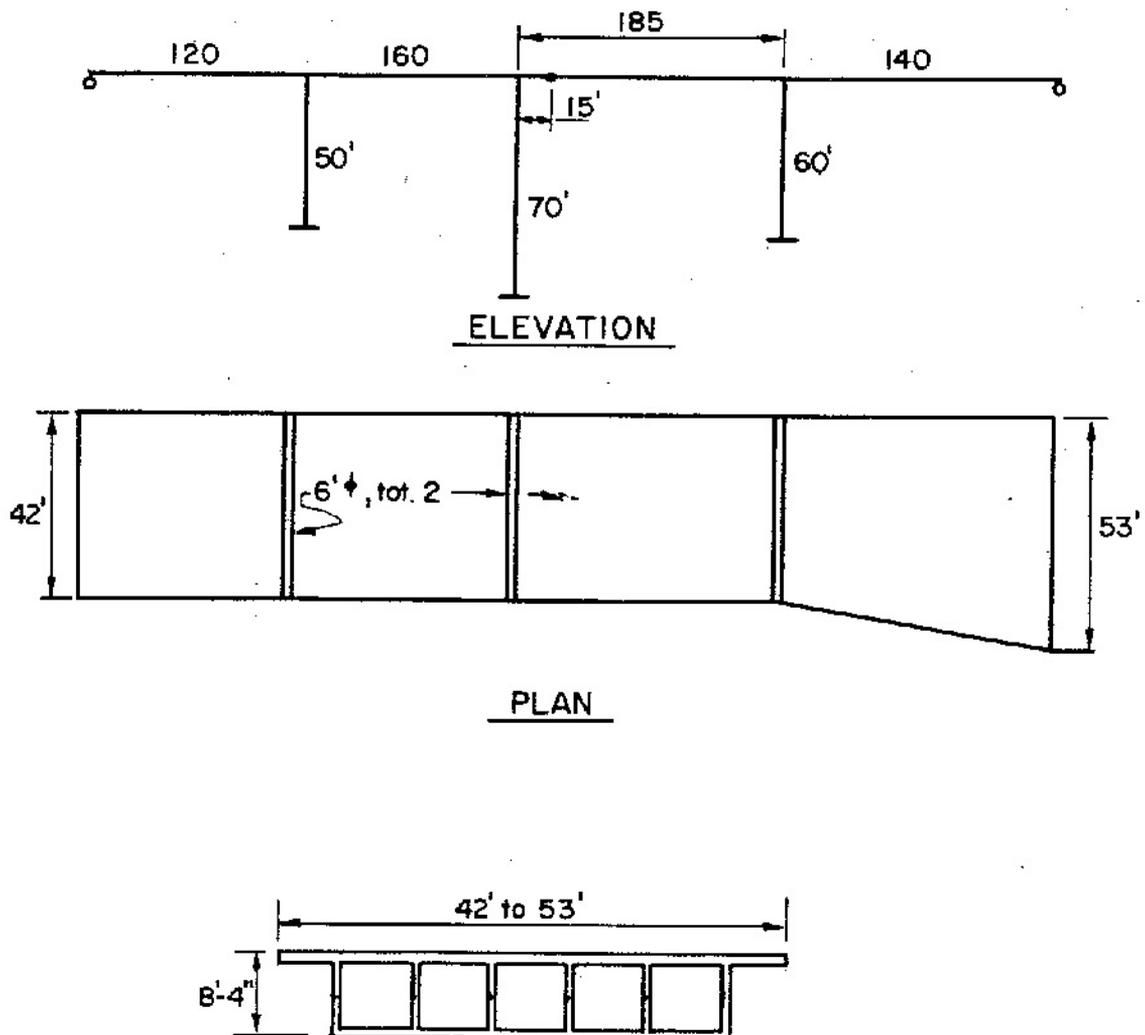
On FRAME DESCRIPTION input, if UNIT WT. is given (accidentally) and the section data (area) was not, the unit wt. is not used and a warning message is printed.

When using SECTION PROPERTIES by PARTS input, if the PART CODE is omitted and data given for AREA and  $I_{xx}$ , but not for the PART DIMENSIONS V and H, a message is printed stating that part code 26 (any shape) was assumed.

If the LOAD DATA dimensions A or B happen to be greater than the member length, the latter is assumed for these dimensions and a message is printed.

The Dead Load analysis (TRIAL NO. 0) will now be performed even if all horizontal members are not loaded. An appropriate message is printed. The analysis performed with this inconsistency will not be added to Live Load results.

EXAMPLE



### FRAME SYSTEM Frame Description

Page 1 of 4

AGENCY	
14T	2000

SOURCE		CHARGE		EXPENDITURE AUTHORIZATION		SPECIAL RESIGNATION WHEN APPLICABLE		PROGRAM NUMBER	
14033	14033	9100026	83414						

Name \_\_\_\_\_  
Phone \_\_\_\_\_

MEMBER NO.	ELEM. NO.	L3	L2	L1	Length	Min. I	Hinge Location	E	Dead Load		Member Properties				Recall	D.L.
									Unit (K/L)	WT.	-K-		-C-			
											Stiffness Factor	Dist. Dist. Factor	Stiffness Factor	Dist. Dist. Factor		
1	1	2	R	H	12.00				360	150						
2	2	3	H		14.00											
3	3	4	H		18.50		150									
4	4	5	H		14.00											
5	6	2	H		5.00	12.724										
6	7	3	H		7.00											
7	8	4	H		6.00											



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### FRAME SYSTEM Superstructure Sections

Page 2 of 4

AGENCY	
14T	2000

SOURCE		CHARGE		EXPENDITURE AUTHORIZATION		SPECIAL RESIGNATION WHEN APPLICABLE		PROGRAM NUMBER	

Name \_\_\_\_\_  
Phone \_\_\_\_\_

MEMBER NO.	DECK SECTION LOCATION (FT.)	NO. OF LANE	DEK. HT. COORD.		S.A. DATA		SLAB DATA		HT. BARRIER	EXTERIOR DIMENSIONS				OVERHEADS				SPAN			
			X (FT.)	Y (FT.)	WIDTH (FT.)	DEPTH (FT.)	TOP FINISH (IN.)	BOTTOM FINISH (IN.)		LEFT		RIGHT		LEFT		RIGHT					
										TYPE	THICK. (IN.)	TYPE	THICK. (IN.)	TYPE	THICK. (IN.)	TYPE	THICK. (IN.)				
01					420	833	663	580	412					40		40				01	
02		01																			
03		02																			
04					420	833	750	600	412					40		40					
04	1400				530	833	750	600	412					40		40					



Form 1-70 8-10

### FRAME SYSTEM Frame Description

Page 1 of 4

AGENCY	
14T	2000

SOURCE		CHARGE		EXPENDITURE AUTHORIZATION		SPECIAL RESIGNATION WHEN APPLICABLE		PROGRAM NUMBER	
14033	14033	9100026	83414						

Name \_\_\_\_\_  
Phone \_\_\_\_\_

MEMBER NO.	ELEM. NO.	L3	L2	L1	Length	Min. I	Hinge Location	E	Dead Load		Member Properties				Recall	D.L.
									Unit (K/L)	WT.	-K-		-C-			
											Stiffness Factor	Dist. Dist. Factor	Stiffness Factor	Dist. Dist. Factor		
1	1	2	R	H	12.00				360	150						
2	2	3	H		14.00											
3	3	4	H		18.50		150									
4	4	5	H		14.00											
5	6	2	H		5.00	12.724										
6	7	3	H		7.00											
7	8	4	H		6.00											

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### FRAME SYSTEM Superstructure Sections

Page 2 of 4

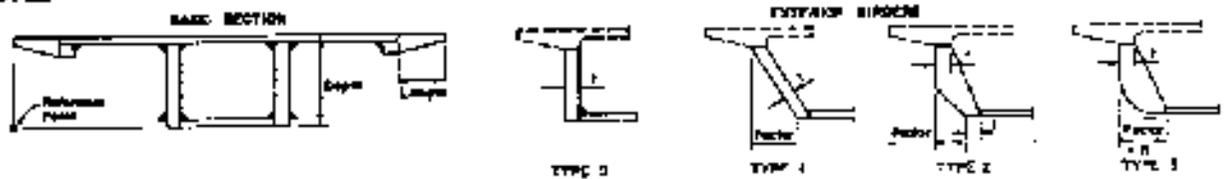
AGENCY	
14T	2000

SOURCE		CHARGE		EXPENDITURE AUTHORIZATION		SPECIAL RESIGNATION WHEN APPLICABLE		PROGRAM NUMBER	

Name \_\_\_\_\_  
Phone \_\_\_\_\_

MEMBER NO.	ELEM. NO.	L3	L2	L1	REQ. BY COORD.		S.A. DATA		SLAB DATA		HT. BARS	EXTERNAL DIMENSIONS				OVERHEADS				SPAN	
					X	Y	WIDTH	DEPTH	TOP THICK.	BOTTOM THICK.		LEFT		RIGHT		LEFT		RIGHT			
												TYPE	THICK.	TYPE	THICK.	TYPE	THICK.	TYPE	THICK.		
01						420	833	663	580	412					40		40			01	
02	01																				
03	01																				
04						420	833	750	600	412					40		40				
04	1400					530	833	750	600	412					40		40				

FORM 4-80 8-10



FORM 4-80 8-10

IDENT		14T 20 00		FRAME SYSTEM							JUL. 22, 1971				PAGE 1	
FRAME DESCRIPTION																
MEM NO	JY LT	JT RT	END COND	DIR	SPAN	I	HINGE	E	DEAD LOAD UNIFORM	SEC	K LT	K RT	CARRY OVER FACTORS LT	RT	RECALL MEM	
1	1	2	R	H	120.0	C.0	0.0	0.	0.360	150.	0.0	0.0	0.0	0.0		
2	2	3		H	140.0	C.0	0.0	0.	0.360	150.	0.0	0.0	0.0	0.0		
3	3	4		H	185.0	0.0	15.0	0.	0.360	150.	0.0	0.0	0.0	0.0		
4	4	5	R	H	140.0	0.0	0.0	0.	0.360	150.	0.0	0.0	0.0	0.0		
5	6	2			50.0	127.24	0.0	0.	0.0	0.	0.0	0.0	0.0	0.0		
6	7	3			70.0	127.24	0.0	0.	0.0	0.	0.0	0.0	0.0	0.0		
7	8	4			60.0	127.24	0.0	0.	0.0	0.	0.0	0.0	0.0	0.0		

IDENT		14T 20 00		FRAME SYSTEM							JUL. 22, 1971				PAGE 2						
SECTION PROPERTIES																					
MEM	LOC	RECALL	X	Y	SUPERSTRUCTURE WIDTH	DEPTH	SLAB THICKNESS TOP	BOTTOM	INT. GIRDER NO.	WEB	STORE	LT. EXT. GIRDER TYPE	WEB FACTOR	RT. EXT. GIRDER TYPE	WEB FACTOR	LT. OVERHANG LENGTH	EXT. INT.	RT. OVERHANG LENGTH	EXT. INT.		
1	0.0		0.0	0.0	42.0	8.33	6.63	5.50	4	12.	01	0	12.	0	12.	4.0	7.	11.	4.0	7.	11.
*** SOME OF THE ABOVE DATA HAS BEEN ASSUMED. ***																					

AREA	CENTROID LOCATION		MOMENT OF INERTIA ABOUT CENTROID	
	X	Y	X-X	Y-Y
85.39	21.00	4.54	810.34	11138.74

MEMBER 1 PROPERTIES															
LENGTH	MIN INERTIA		STIFFNESS		CARRY OVER										
	LT	RT	LT	RT	LT	RT									
120.0			4.000	4.000	0.500	0.500									

IDENT		14T 20 00		FRAME SYSTEM							JUL. 22, 1971				PAGE 3					
SECTION PROPERTIES																				
MEM	LOC	RECALL	X	Y	SUPERSTRUCTURE WIDTH	DEPTH	SLAB THICKNESS TOP	BOTTOM	INT. GIRDER NO.	WEB	STORE	LT. EXT. GIRDER TYPE	WEB FACTOR	RT. EXT. GIRDER TYPE	WEB FACTOR	LT. OVERHANG LENGTH	EXT. INT.	RT. OVERHANG LENGTH	EXT. INT.	
2	0.0	01	0.0	C.0	0.0	0.0	C.C	0.0	0	0.		0	0.	0	0.	0.0	0.	0.0	0.	0.
INERTIAS OF SECTION PARTS																				

MEM	LOC	RECALL	SIGN CODE	V	H	X	Y	AREA	IXX	IYY	STORE
1						21.00	4.54	85.39	810.34	11138.74	
AREA		CENTROID LOCATION		MOMENT OF INERTIA ABOUT CENTROID							
	X	Y	X-X	Y-Y							
85.39	21.00	4.54	810.34	11138.78							

MEMBER 2 PROPERTIES															
LENGTH	MIN INERTIA		STIFFNESS		CARRY OVER										
	LT	RT	LT	RT	LT	RT									
140.0			4.000	4.000	0.500	0.500									



IDENT 14T 20 00

FRAME SYSTEM

JUL. 22, 1971

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FRAME DIAGNOSTICS

FRAME PROPERTIES

MEM NO	JT LT	JT RT	END COND		DIR	SPAN	I	HINGE	E	CARRY OVER FACTORS		DISTRIBUTION FACTORS	
			LT	RT						LT	RT		
1	1	2	R		H	120.0	810.34	0.0	750.	0.500	0.0	0.0	0.400
2	2	3			H	160.0	810.34	0.0	750.	0.500	0.500	0.400	0.733
3	3	4			H	185.0	810.34	15.0	750.	11.333	0.088	0.004	0.341
4	4	5	R		H	140.0	849.93	0.0	750.	0.0	0.476	0.457	0.0
5	6	2				50.0	127.24	0.0	750.	0.500	0.500	0.0	0.201
6	7	3				70.0	127.24	0.0	750.	0.500	0.500	0.0	0.263
7	8	4				60.0	127.24	0.0	750.	0.500	0.500	0.0	0.202

IDENT 14T 20 00

FRAME SYSTEM

JUL. 22, 1971

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LOAD DATA TRIAL 0

MEM	W OR P	LOAD CODE	A	B	FIXED END MOMENTS			COMMENTS
					LEFT	RIGHT	DEFLT	
1	20.500	P	60.0	0.0	0.	0.	DIAPH	
1	1.400	U	0.0	0.0	0.	0.	AC SURFACING	
			ASSUMED DATA 120.0					
2	1.400	U	0.0	0.0	0.	0.	AC SURFACING	
			ASSUMED DATA 140.0					
2	20.500	P	80.0	0.0	0.	0.	DIAPH	
3	20.500	P	100.0	0.0	0.	0.	DIAPH	
3	1.400	U	0.0	0.0	0.	0.	AC SURFACING	
			ASSUMED DATA 185.0					
3	122.600	P	15.0	0.0	0.	0.	HINGE	
4	24.600	P	70.0	0.0	0.	0.	DIAPH	
4	1.400	U	0.0	0.0	0.	0.	AC SURFACING	
			ASSUMED DATA 140.0					
4	0.385	R	0.0	0.0	0.	0.	AC SURFACING	
			ASSUMED DATA 140.0					

FIXED END MOMENTS TRIAL 0

MEM NO	FIXED END MOMENTS LT	FIXED END MOMENTS RT	MEM NO	FIXED END MOMENTS LT	FIXED END MOMENTS RT	MEM NO	FIXED END MOMENTS LT	FIXED END MOMENTS RT
1	0.	-26685.	2	-31490.	-31490.	3	-17494.	-53416.
4	-38859.	0.	5	0.	0.	6	0.	0.
7	0.	0.						

IDENT 14T 20 00

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SIDESWAY DIAGNOSTICS

RESULTS OF 1 INCH SWAY TO THE RIGHT

VERTICAL MEMBER	SHEAR (KIPS)	MOMENTS (FT-KIPS)	
		LT	RT
5	95.4	-2506.	2244.
6	34.9	-1281.	1140.
7	54.0	-1715.	1522.



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TANGENTIAL ROTATIONS - HANSONS - CLOCKWISE POSITIVE

SPAN	LT. END	RT. END	SPAN	LT. END	RT. END	SPAN	LT. END	RT. END
1	0.005179	0.001839	2	0.001839	-0.005287	3	-0.005287	-0.003600
4	-0.003600	-0.007571	5	-0.000000	0.001839	6	0.000000	-0.005287
7	-0.000000	-0.003600						

HORIZONTAL MEMBER DEFLECTIONS IN FEET AT 1/4 POINTS FROM LEFT END - DOWNWARD POSITIVE

MEMBER	END	SPAN	1/4	1/2	3/4	RT	
MEMBER 1	E= 750.	0.0	0.129	0.142	0.090	0.0	
MEMBER 2	E= 750.	0.0	0.243	0.432	0.295	0.0	
MEMBER 3	E= 750.	0.0	0.499	0.845	0.487	0.0	
LONG HINGE			LT	1/4	1/2	3/4	RT
			-0.089	0.650	0.829	0.439	0.0
MEMBER 4	E= 750.	0.0	0.079	0.242	0.220	0.0	

VERTICAL MEMBER DEFLECTIONS IN FEET AT 1/4 POINTS FROM LEFT END.

MEMBER	END	SPAN	1/4	1/2	3/4	RT
MEMBER 5	E= 750.	0.0	-0.007	-0.019	-0.028	-0.016
MEMBER 6	E= 750.	0.0	0.015	0.018	0.039	-0.016
MEMBER 7	E= 750.	0.0	-0.007	-0.027	-0.041	-0.108

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LIVE LOAD DIAGNOSTICS

SUPERSTRUCTURE LIVE LOAD EMPTY CREEK BRIDGE

MEM NO.	NUMBER OF LIVE LOAD LINES				RESISTING MOMENT OF UNIT STEEL		PLOT MOMENT ENV.	PLOT SHEAR ENV.	INFLUENCE LINES
	LT. END	RT. END	LT. END	RT. END	POSITIVE	NEGATIVE			
1	3.000	3.000	2.7	2.7	0.	0.	NC	NC	NO
2	3.000	3.000	2.7	2.7	0.	0.			
3	3.000	3.000	2.7	2.7	0.	0.			
4	3.000	3.706	2.7	3.0	0.	0.			

LIVE LOAD NO.	TRUCK					LANE			IMPACT	MUL. LNS.	LIVE LOAD SIDESWAY	COMMENTS
	P1	P1	P2	P2	P3	UNIFORM	MEM. RIDER	SPEAR RIDE				
1.	0.0	14.0	32.0	14.0	37.0	0.440	38.0	26.0	YES	0.0	NO	HS20-44 BRSD LOADING WITHOUT ALTERNATIVE
2.	70.0	33.0	130.0	28.0	130.0	0.0	0.0	0.0	YES	1.00	NO	CONSTRUCTION LOAD

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LL NO. 1. NEGATIVE LIVE LOAD MOMENT ENVELOPE

MEM NO.	LEFT	1 PT	2 PT	3 PT	4 PT	5 PT	6 PT	7 PT	8 PT	9 PT	RIGHT
1	0.	-185.	-732.	-1096.	-1461.	-1826.	-2191.	-2556.	-2921.	-3286.	-3651.
2	-7479.	-3946.	-1811.	-1467.	-1134.	-801.	-468.	-138.	174.	539.	904.
3	-9575.	-57.	-358.	-699.	-1061.	-1422.	-1783.	-2144.	-2505.	-2866.	-3227.
4	-9018.	-5694.	-3778.	-3249.	-2877.	-2506.	-2135.	-1764.	-1393.	-1022.	-651.

IDENT 14T 20 00		FRAME SYSTEM									JUL. 22, 1971		PAGE 15
LL NO. 1.		DEAD LOAD PLUS NEGATIVE LIVE LOAD MOMENT ENVELOPE											
MEM NO	LEFT	.1 PT	.2 PT	.3 PT	.4 PT	.5 PT	.6 PT	.7 PT	.8 PT	.9 PT	RIGHT		
1	0.	6127.	10157.	12089.	11922.	9658.	5050.	-1656.	-10516.	-22511.	-37579.		
2	-40729.	-19117.	-2633.	8331.	15114.	18167.	17162.	12429.	3965.	-8458.	-26610.		
3	-21623.	3253.	17485.	26730.	30989.	30262.	24323.	13244.	-3026.	-26406.	-56633.		
4	-55330.	-33168.	-15484.	-2234.	7926.	14961.	18482.	18791.	15843.	9594.	-0.		

IDENT 14T 20 00		FRAME SYSTEM									JUL. 22, 1971		PAGE 16
LL NO. 1.		POSITIVE LIVE LOAD MOMENT ENVELOPE											
MEM NO	LEFT	.1 PT	.2 PT	.3 PT	.4 PT	.5 PT	.6 PT	.7 PT	.8 PT	.9 PT	RIGHT		
1	0.	2416.	4062.	5002.	5370.	5216.	4608.	3549.	2172.	893.	564.		
2	847.	989.	2617.	4269.	5494.	6125.	6146.	5488.	4114.	2084.	455.		
3	240.	788.	4167.	6346.	7529.	7676.	6829.	5073.	3103.	1000.	242.		
4	167.	978.	2940.	4770.	6243.	7174.	7409.	7014.	5752.	3474.	0.		

IDENT 14T 20 00		FRAME SYSTEM									JUL. 22, 1971		PAGE 17
LL NO. 1.		DEAD LOAD PLUS POSITIVE LIVE LOAD MOMENT ENVELOPE											
MEM NO	LEFT	.1 PT	.2 PT	.3 PT	.4 PT	.5 PT	.6 PT	.7 PT	.8 PT	.9 PT	RIGHT		
1	0.	8909.	14950.	18157.	18753.	16702.	11850.	4450.	-5366.	-17181.	-30145.		
2	-32402.	-14181.	1794.	14067.	22181.	25972.	25096.	19811.	10081.	-4035.	-21475.		
3	-17807.	4098.	22010.	33736.	39479.	39200.	32715.	20183.	2449.	-20404.	-46858.		
4	-46147.	-26536.	-8767.	5834.	16996.	24493.	27777.	27219.	22538.	13539.	-0.		



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FRAME SYSTEM

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LL NO. 1.

## LIVE LOAD SUPPORT RESULTS

	MAX. AXIAL LOAD	SXIAL LOAD -----MOMENT-----		MAX. AXIAL LOAD	LONGITUDINAL MCMENT -----PCMENT-----	
		TOP	BOT.		TOP	ROT.
SUPPORT JT. 1						
POSITIVE	210.8	0.	0.	0.0	0.	0.
NEGATIVE	-30.0	0.	0.	0.0	0.	0.
MEMBER 5						
POSITIVE	436.3	-484.	242.	180.0	1291.	-646.
NEGATIVE	-26.0	285.	-142.	244.3	-1651.	825.
MEMBER 6						
POSITIVE	423.4	459.	-230.	169.4	1946.	-973.
NEGATIVE	-26.0	-279.	140.	237.6	-1125.	562.
MEMBER 7						
POSITIVE	486.3	461.	-230.	268.7	1876.	-938.
NEGATIVE	-2.6	-73.	37.	222.9	-1411.	705.
SUPPORT JT. 5						
POSITIVE	233.9	0.	0.	0.0	0.	0.
NEGATIVE	-33.0	0.	0.	0.0	0.	0.

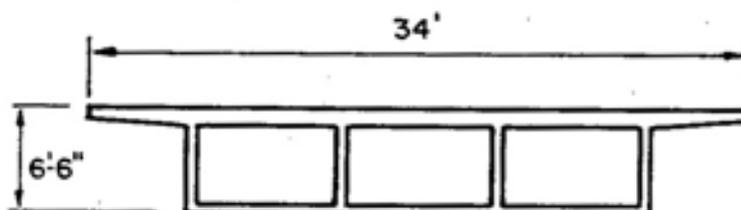
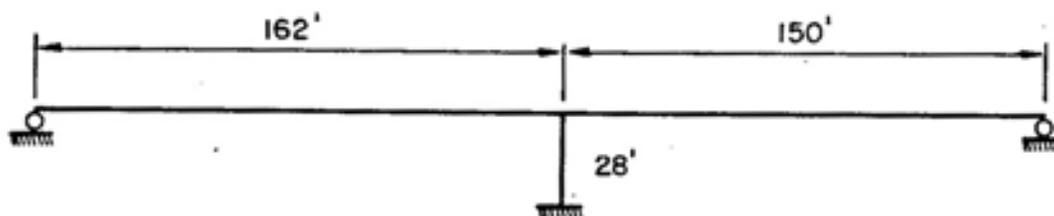
Results for L. L. No. 2 are similar and are not shown in this example.

### MOMENT AND SHEAR PLOTTED RESULTS

Dead load moments and shears are plotted as dashed lines. Live load moments and shears are plotted as solid lines from enveloped data as presented in the printed output. Plots will be annotated to show if the Dead Load is included or excluded from the Live Load envelopes.

### INFLUENCE LINE PLOTTED RESULTS

When influence lines are requested the plotted results will produce separate plot frames for each horizontal member. Each plot frame will contain the influence lines for the .2L, .4L, .5L, .6L, .8L, and 1.0L positions along the particular member. Each influence line is delineated by a separate line code shown on the plot.



### FRAME SYSTEM Frame Description

BOEDRA

Page 1 of 4

Name EPED FRAME

Phone \_\_\_\_\_

IDENT.		SOURCE		CHARGE		TEMPERATURE AUTHORIZATION		SPECIAL DESIGNATION AND APPLICATION		PROGRAM NUMBER	
NO.	DATE	NO.	DATE	NO.	DATE	NO.	DATE	NO.	DATE	NO.	DATE
14T	0501	14033	14033	910002						S. S. C. R. 3.2.5	
A/C 2881, 1518		A/C 2881									

Update Lay No.	Member No.	Elem No.	Elem Type	Divi- sion	Length	Min I	Node Location	E	Deck Level		Material Properties				Rebar	E <sub>u</sub>
									A/R	Wt	-R-		-D-			
											LL	RI	LL	RI		
	1	1	2R		1620				150							
	2	2	3R		1500				150							
	3	4	2		280	8400										



FORM R-80 (REV. 8-6-72)

### FRAME SYSTEM Superstructure Sections

BOEDRA

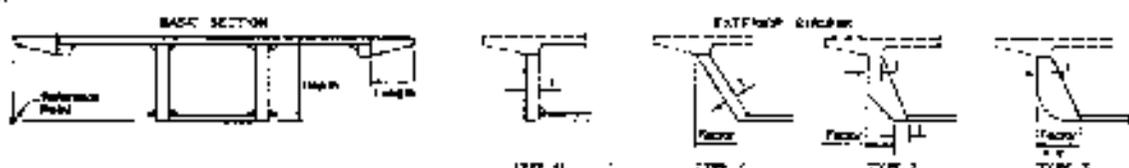
Page 2 of 4

Name \_\_\_\_\_

Phone \_\_\_\_\_

IDENT.		SOURCE		CHARGE		TEMPERATURE AUTHORIZATION		SPECIAL DESIGNATION AND APPLICATION		PROGRAM NUMBER	
NO.	DATE	NO.	DATE	NO.	DATE	NO.	DATE	NO.	DATE	NO.	DATE
14T	0501									S. S. C. R. 3.2.5	
A/C 2881, 1518		A/C 2881									

UPDATE Lay No.	MEMBER NO.	ELEM NO.	ELEM TYPE	REF. COORD.	S.S. DATA		SLAB DATA		EX. BY RICH. GINOP		OVERHANGS		
					PC-1	PC-2	TOP THICK	BOTTOM THICK	TYPE	THICK	TYPE	THICK	
	1	00		00.00	340	650	675	575	2120	12	95	71135	71101
				1193701									
				11620	340	650	675	1200	2120	12	85	71135	71102
	2	0002											
				218301									
				2150001									



Form R-80 (REV. 8-6-72)

FRAME SYSTEM

Load Data

BDE0AA

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Name \_\_\_\_\_

Phone \_\_\_\_\_

IDENT.			
DEPT.	NO.	BATCH	PLAN
14T	0501		
S/C 7318			

\* CODE  
 L: Max. W on left  
 R: Max. W on right  
 U: Uniform Load  
 P: Point Load



Update	Line No.	Title No.	Member No.	Loads			FEMs*		Deflections	Comments
				W or P k/ft or k	A ft	B ft	Left ft-k	Right ft-k		
		01		14.037P	81.0					DIAPH
		02		14.037P	75.0					DIAPH

S/C 7318

\*When FEMs are given, they are not calculated for any load on that member.

FORM 1-68 (REV. 6/72)

FRAME SYSTEM  
 Superstructure Live Load

BDE0AA

Page 4 of 4

Name \_\_\_\_\_

Phone \_\_\_\_\_

IDENT.			
DEPT.	NO.	BATCH	PLAN
14T	0501		
S/C 7320, 7321			

MEMBER DATA

Update	Line No.	Member No.	Number of Live Load Lanes				Plot Data			COMMENTS
			Superstructure		Substructure		Resisting Moment of Unit Steel		Influence Lines	
			Lt. End	Rt. End	Lt.	Rt.	Positive	Negative		
		1	2430	2430	10	10	4500	5000	1	1/2 SPAN EXAMPLE
		2	2430	2430	10	10	4500	5000		

S/C 7320

Frame Description data with the horizontal members numbered consecutively starting with 01 must accompany this data.

Member Data - When the Number of L.L. Lanes is given, it must be given for the left end of Superstructure Member 01. (Substructure Member 01 defaults to 2.0 when left blank.) Thereafter, it is assumed to be constant until another entry is made.

Live Load Data - For AASHTO HS20-44 loading, leave Truck and Lane data blank for L.L. No. 1. When this data is given, it replaces the HS20-44 loading. An entry for the Number of Live Load Lanes, overrides that given as Member Data. Data entries for L.L. No.'s 2 and 3 produce separate results in addition to L.L. No. 1.

Influence Lines - When checked, a plot of the influence lines will be produced along with the printed results.

LIVE LOAD DATA

Update	Line No.	L.L. No.	Truck - (1 Lane)					Lane - (2 Lane)			Number of Live Load Lanes	COMMENTS
			P <sub>1</sub> Kips	D <sub>1</sub> ft	P <sub>2</sub> Kips	D <sub>2</sub> ft	P <sub>3</sub> Kips	Uniform Kips/ft	Moment Rider Kips	Shear Rider Kips		
		1										
		2										
		3										

S/C 7321







IDENT 14T 05 01 FRAM SYSTEM DEC. 13, 1972 PAGE 10

LIVE LOAD DIAGNOSTICS

NO ERRORS FOUND

SUPERSTRUCTURE LIVE LOAD 2 SPAN EXAMPLE

LINE NO.	MEM NO.	NUMBER OF LIVE LOAD LANES				RESISTING MOMENT OF UNIT STEEL		PLOT NO.	PLOT SCALE	INFLUENCE LINES
		LT.END	RT.END	LT.END	RT.END	POSITIVE	NEGATIVE			
	1	2.430	2.430	1.0	1.0	4500.	5000.	1	0	YES
	2	2.430	2.430	1.0	1.0	4500.	5000.			

LIVE LOAD NO.	TRUCK					LANE			NO. LNS.	LIVE LOAD SIDESWAY	COMMENTS
	P1	D1	P2	D2	P3	UNIFORM	MOM. RIDLR	SHEAR RIDLR			
1-	8.0	14.0	32.0	14.0	32.0	0.440	18.0	28.0	YES	0.0	MS20-44 AASHO LOADING WITHOUT ALTERNATIVE

IDENT 14T 05 01 FRAM SYSTEM DEC. 13, 1972 PAGE 11

INFLUENCE LINES FOR GIRDLR MOMENT

AT 0.1 L - MEMBER 1

MEM NO	LEFT	-1 PT	-2 PT	-3 PT	-4 PT	-5 PT	-6 PT	-7 PT	-8 PT	-9 PT	RIGHT
1	0.0	13.966	11.770	9.668	7.638	5.777	4.102	2.652	1.462	0.571	0.0
2	0.0	-0.336	-0.570	-0.708	-0.762	-0.745	-0.668	-0.543	-0.362	-0.197	0.0

AT 0.2 L - MEMBER 1

MEM NO	LEFT	-1 PT	-2 PT	-3 PT	-4 PT	-5 PT	-6 PT	-7 PT	-8 PT	-9 PT	RIGHT
1	0.0	11.732	23.539	19.295	15.275	11.553	8.205	5.393	2.925	1.142	0.0
2	0.0	-0.672	-1.140	-1.416	-1.525	-1.490	-1.336	-1.086	-0.764	-0.394	0.0

AT 0.3 L - MEMBER 1

MEM NO	LEFT	-1 PT	-2 PT	-3 PT	-4 PT	-5 PT	-6 PT	-7 PT	-8 PT	-9 PT	RIGHT
1	0.0	9.498	19.109	16.943	12.913	17.330	12.307	7.955	4.387	1.714	0.0
2	0.0	-1.001	-1.710	-2.124	-2.287	-2.235	-2.004	-1.628	-1.145	-0.591	0.0

AT 0.4 L - MEMBER 1

MEM NO	LEFT	-1 PT	-2 PT	-3 PT	-4 PT	-5 PT	-6 PT	-7 PT	-8 PT	-9 PT	RIGHT
1	0.0	7.265	14.678	22.390	30.550	15.107	16.409	16.607	5.849	2.285	0.0
2	0.0	-1.344	-2.279	-2.882	-3.050	-2.960	-2.672	-2.171	-1.527	-0.788	0.0

AT 0.5 L - MEMBER 1

MEM NO	LEFT	-1 PT	-2 PT	-3 PT	-4 PT	-5 PT	-6 PT	-7 PT	-8 PT	-9 PT	RIGHT
1	0.0	5.031	16.246	15.838	21.948	16.683	20.512	19.259	7.311	2.804	0.0
2	0.0	-1.680	-2.849	-3.540	-3.812	-3.725	-3.339	-2.714	-1.909	-0.984	0.0

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INFLUENCE LINES FOR GIRDLR MOMENT

AT 0.6 L - MEMBER 1

MEM NO	LEFT	-1 PT	-2 PT	-3 PT	-4 PT	-5 PT	-6 PT	-7 PT	-8 PT	-9 PT	RIGHT
1	0.0	2.797	5.817	9.286	13.425	16.460	24.614	15.910	8.774	3.427	0.0
2	0.0	-2.016	-3.419	-4.248	-4.575	-4.471	-4.007	-3.257	-2.291	-1.181	0.0

AT 0.7 L - MEMBER 1

MEM NO	LEFT	-1 PT	-2 PT	-3 PT	-4 PT	-5 PT	-6 PT	-7 PT	-8 PT	-9 PT	RIGHT
1	0.0	0.343	1.387	2.733	4.463	8.037	12.514	18.562	10.234	3.998	0.0
2	0.0	-2.352	-3.989	-4.936	-5.337	-5.216	-4.675	-3.800	-2.673	-1.378	0.0

AT 0.8 L - MEMBER 1

MEM NO	LEFT	-1 PT	-2 PT	-3 PT	-4 PT	-5 PT	-6 PT	-7 PT	-8 PT	-9 PT	RIGHT
1	0.0	-1.671	-3.043	-3.819	-3.700	-2.386	0.419	5.014	11.698	4.969	0.0
2	0.0	-2.688	-4.559	-5.664	-6.100	-5.961	-5.343	-4.342	-3.054	-1.575	0.0

AT 0.9 L - MEMBER 1

MEM NO	LEFT	-1 PT	-2 PT	-3 PT	-4 PT	-5 PT	-6 PT	-7 PT	-8 PT	-9 PT	RIGHT
1	0.0	-3.905	-7.474	-10.371	-12.262	-12.810	-11.679	-8.534	-3.040	5.141	0.0
2	0.0	-3.024	-5.129	-6.373	-6.862	-6.706	-6.011	-4.885	-3.436	-1.772	0.0

AT 1.0 L - MEMBER 1

MEM NO	LEFT	-1 PT	-2 PT	-3 PT	-4 PT	-5 PT	-6 PT	-7 PT	-8 PT	-9 PT	RIGHT
1	0.0	-6.139	-11.964	-16.924	-20.824	-23.233	-23.777	-22.083	-17.777	-10.488	0.0
2	0.0	-3.359	-5.699	-7.041	-7.625	-7.451	-6.679	-5.428	-3.818	-1.989	0.0



IDENT 14T 05 01		FRAME SYSTEM										DEC. 13, 1972	PAGE 15
INFLUENCE LINES FOR GIRDER SHEAR													
AT LEFT END - MEMBER 1													
MEM NO	LEFT	-1 PT	-2 PT	-3 PT	-4 PT	-5 PT	-6 PT	-7 PT	-8 PT	-9 PT	RIGHT		
1	1.000	0.862	0.727	0.596	0.471	0.357	0.253	0.164	0.090	0.035	0.0		
2	0.0	-0.021	-0.035	-0.044	-0.047	-0.046	-0.041	-0.034	-0.024	-0.012	0.0		
AT LEFT END - MEMBER 2													
MEM NO	LEFT	-1 PT	-2 PT	-3 PT	-4 PT	-5 PT	-6 PT	-7 PT	-8 PT	-9 PT	RIGHT		
1	0.0	0.015	0.030	0.042	0.052	0.058	0.059	0.055	0.044	0.026	0.0		
2	1.000	0.963	0.907	0.833	0.743	0.640	0.525	0.402	0.272	0.137	0.0		

IDENT 14T 05 01		FRAME SYSTEM										DEC. 13, 1972	PAGE 16
INFLUENCE LINES													
REACTION AT LT END MEMBER 1													
MEM NO	LEFT	-1 PT	-2 PT	-3 PT	-4 PT	-5 PT	-6 PT	-7 PT	-8 PT	-9 PT	RIGHT		
1	1.000	0.862	0.727	0.596	0.471	0.357	0.253	0.164	0.090	0.035	0.0		
2	0.0	-0.021	-0.035	-0.044	-0.047	-0.046	-0.041	-0.034	-0.024	-0.012	0.0		
REACTION AT TOP OF COLUMN 3													
MEM NO	LEFT	-1 PT	-2 PT	-3 PT	-4 PT	-5 PT	-6 PT	-7 PT	-8 PT	-9 PT	RIGHT		
1	0.0	0.153	0.303	0.447	0.580	0.701	0.806	0.891	0.954	0.991	1.000		
2	1.000	0.984	0.942	0.876	0.790	0.686	0.566	0.435	0.295	0.149	0.0		
MOMENT AT TOP OF COLUMN 3													
MEM NO	LEFT	-1 PT	-2 PT	-3 PT	-4 PT	-5 PT	-6 PT	-7 PT	-8 PT	-9 PT	RIGHT		
1	0.0	3.841	7.449	10.591	13.031	14.539	14.879	13.819	11.325	6.563	0.0		
2	0.0	-6.079	-10.313	-12.613	-13.798	-13.463	-12.086	-9.823	-6.909	-3.563	0.0		
REACTION AT RT END MEMBER 2													
MEM NO	LEFT	-1 PT	-2 PT	-3 PT	-4 PT	-5 PT	-6 PT	-7 PT	-8 PT	-9 PT	RIGHT		
1	0.0	-0.015	-0.030	-0.042	-0.052	-0.058	-0.059	-0.055	-0.044	-0.026	0.0		
2	0.0	0.037	0.093	0.167	0.257	0.360	0.475	0.598	0.728	0.863	1.000		

IDENT 14T 05 01		FRAME SYSTEM										DEC. 13, 1972	PAGE 17
LL NO. 1. NEGATIVE LIVE LOAD MOMENT ENVELOPE													
MEM NO	LEFT	-1 PT	-2 PT	-3 PT	-4 PT	-5 PT	-6 PT	-7 PT	-8 PT	-9 PT	RIGHT		
1	0.	-176.	-352.	-528.	-705.	-881.	-1057.	-1233.	-2046.	-4317.	-7556.		
2	-7118.	-4240.	-2233.	-1519.	-1302.	-1085.	-868.	-651.	-434.	-217.	0.		

IDENT 14T 05 01		FRAME SYSTEM										DEC. 13, 1972	PAGE 18
LL NO. 1.		DEAD LOAD PLUS NEGATIVE LIVE LOAD MOMENT ENVELOPE											
MEM NO	LEFT	.1 PT	.2 PT	.3 PT	.4 PT	.5 PT	.6 PT	.7 PT	.8 PT	.9 PT	RIGHT		
1	0.	7070.	11940.	14610.	15082.	13353.	9198.	2843.	-6347.	-19197.	-35321.		
2	-33006.	-18857.	-7564.	548.	6177.	10121.	11866.	11729.	9705.	5796.	0.		

IDENT 14T 05 01		FRAME SYSTEM										DEC. 13, 1972	PAGE 19
LL NO. 1.		POSITIVE LIVE LOAD MOMENT ENVELOPE											
MEM NO	LEFT	.1 PT	.2 PT	.3 PT	.4 PT	.5 PT	.6 PT	.7 PT	.8 PT	.9 PT	RIGHT		
1	0.	2610.	4336.	5274.	5524.	5219.	4391.	3161.	1720.	471.	0.		
2	0.	439.	1639.	2984.	4120.	4871.	5138.	4694.	4020.	2418.	0.		

IDENT 14T 05 01		FRAME SYSTEM										DEC. 13, 1972	PAGE 20
LL NO. 1.		DEAD LOAD PLUS POSITIVE LIVE LOAD MOMENT ENVELOPE											
MEM NO	LEFT	.1 PT	.2 PT	.3 PT	.4 PT	.5 PT	.6 PT	.7 PT	.8 PT	.9 PT	RIGHT		
1	0.	9856.	16628.	20413.	21310.	19453.	14046.	7237.	-2581.	-14409.	-27763.		
2	-25836.	-14178.	-3692.	5051.	11699.	16076.	17674.	17274.	14159.	8431.	0.		

IDENT 14T 05 01		FRAME SYSTEM										DEC. 13, 1972	PAGE 21
LL NO. 1.		LIVE LOAD SHEAR ENVELOPES											
MEMBER 1	LEFT	.1 PT	.2 PT	.3 PT	.4 PT	.5 PT	.6 PT	.7 PT	.8 PT	.9 PT	RIGHT		
POS.	193.9	161.1	133.8	107.7	83.4	61.2	41.6	25.1	12.2	3.2	0.0		
NEG.	-12.0	-20.7	-40.1	-67.6	-93.8	-118.5	-141.1	-163.9	-195.2	-227.1	-258.8		
RANGE	205.8	181.8	173.9	175.3	177.2	179.7	182.7	189.0	207.4	230.2	258.8		

LL NO. 1.		LIVE LOAD SHEAR ENVELOPES										
MEMBER 2	LEFT	.1 PT	.2 PT	.3 PT	.4 PT	.5 PT	.6 PT	.7 PT	.8 PT	.9 PT	RIGHT	
POS.	249.5	219.7	189.7	160.4	140.1	117.4	92.6	66.2	39.2	23.5	15.8	
NEG.	0.0	-3.2	-12.4	-25.4	-41.9	-61.5	-83.6	-108.0	-134.0	-161.2	-189.2	
RANGE	249.5	222.9	202.0	185.8	182.0	178.8	176.2	174.2	173.2	184.7	205.0	

IDENT 14T 05 01		FRAME SYSTEM										DEC. 13, 1972	PAGE 22
LL NO. 1.		DEAD LOAD PLUS LIVE LOAD SHEAR ENVELOPE											
MEMBER	1	LEFT	.1 PT	.2 PT	.3 PT	.4 PT	.5 PT	.6 PT	.7 PT	.8 PT	.9 PT	RIGHT	
POS.			709.0	540.5	377.4	215.0	55.4	-102.6	-271.9	-424.2	-572.8	-718.1	-873.2
NEG.			503.2	358.7	203.5	40.3	-121.6	-282.2	-454.6	-613.2	-780.3	-948.3	-1132.0

LL NO. 1.		DEAD LOAD PLUS LIVE LOAD SHEAR ENVELOPE											
MEMBER	2	LEFT	.1 PT	.2 PT	.3 PT	.4 PT	.5 PT	.6 PT	.7 PT	.8 PT	.9 PT	RIGHT	
POS.			1073.5	902.0	745.7	590.7	444.8	296.3	131.7	-20.3	-173.1	-314.5	-447.9
NEG.			824.0	679.1	543.7	405.0	202.7	117.4	-44.5	-194.5	-346.3	-499.2	-652.9

IDENT 14T 01 01		FRAME SYSTEM						DEC. 13, 1972	PAGE 23
LL NO. 1.		LIVE LOAD SUPPORT RESULTS							
		MAX. AXIAL LOAD	AXIAL LOAD MOMENT		MAX. AXIAL LOAD	LONGITUDINAL MOMENT			
			TOP	BOT.		TDP	BOT.		
SUPPORT JT. 1	POSITIVE	79.8	0.	0.	0.0	0.	0.		
	NEGATIVE	-4.9	0.	0.	0.0	0.	0.		
MEMBER 3	POSITIVE	176.7	160.	-80.	94.2	1493.	-746.		
	NEGATIVE	0.0	0.	0.	67.5	-1312.	656.		
SUPPORT JT. 3	POSITIVE	77.9	0.	0.	0.0	0.	0.		
	NEGATIVE	-6.5	0.	0.	0.0	0.	0.		

THE RATIO OF SUBSTRUCTURE / SUPERSTRUCTURE LOADING IS 0.412

\*\*\*\*\* BATCH TOTALS      15 FRAME UNITS      2 SUPERSTRUCTURE UNITS      8 PLOT UNITS      COST \* \$      6.70

